



THE STRENGTH WITHIN

# C JOIST SPAN TABLES AND DETAILS



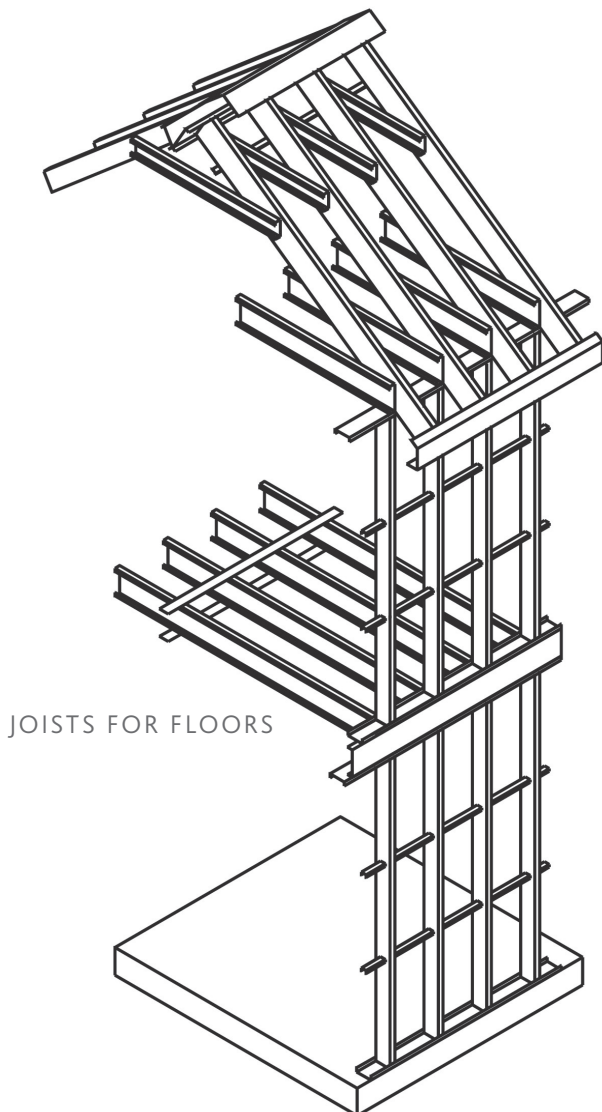
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**BAILEY**<sup>®</sup>  
METAL PRODUCTS LIMITED

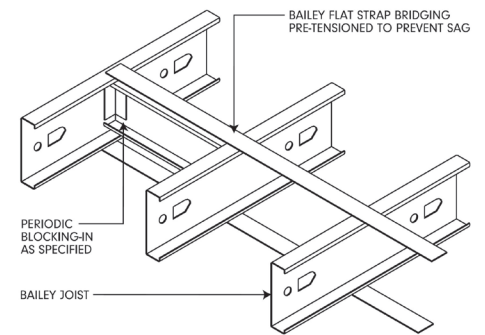


# FLOOR JOISTS

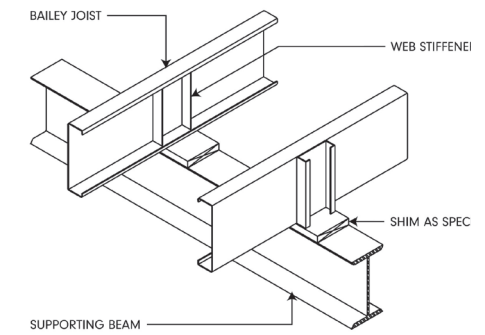
Bailey Lightweight Steel Framing (LSF) Joists offer a wide range of span and load capabilities for commercial and residential floor systems and mezzanines. The use of Bailey LSF Joist Rafter members will provide support for interior drywall ceilings where long clear spans are required.



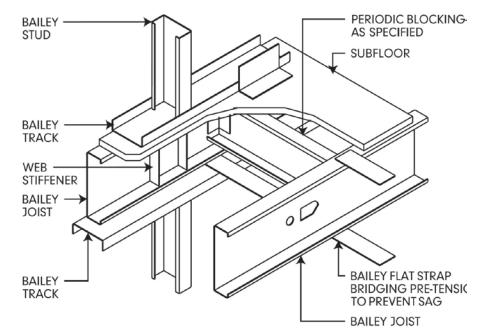
## FLOOR JOIST WITH BRIDGING AND BLOCKING



## JOISTS CONTINUOUS OVER SUPPORTING BEAM



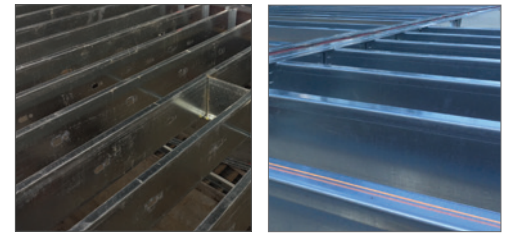
## JOIST PARALLEL TO PERIMETER



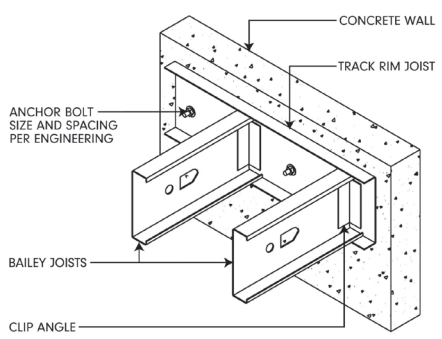


# BAILEY

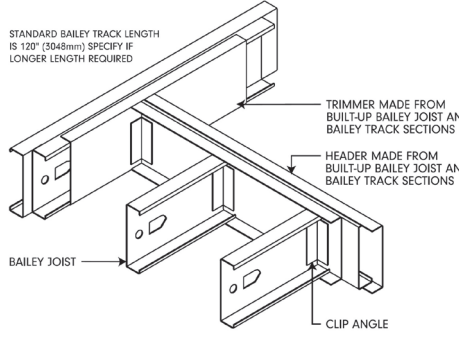
METAL PRODUCTS LIMITED



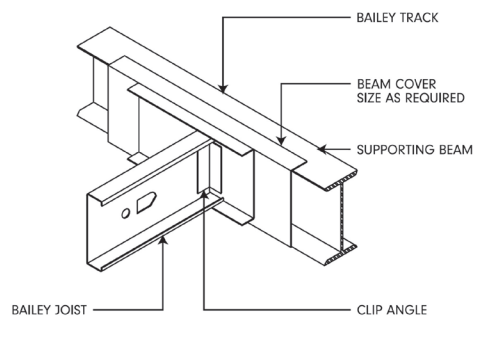
FLOOR MOUNTED INSIDE CONCRETE WALL



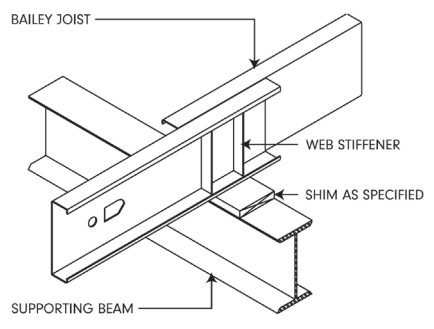
OPENING IN FLOOR



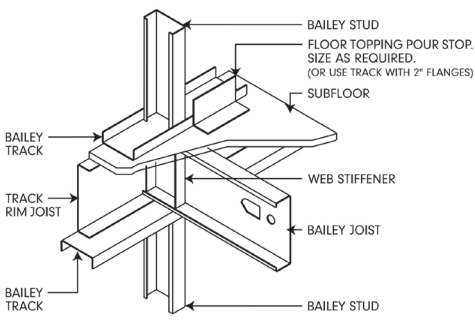
FLUSH CONNECTION TO SUPPORTING BEAM



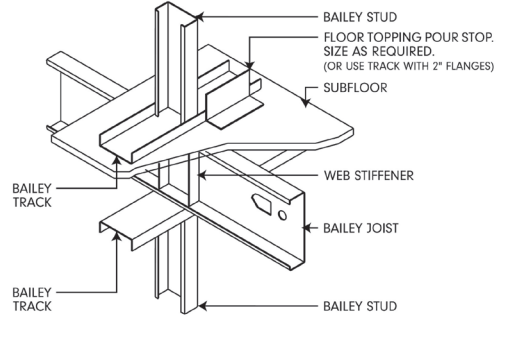
JOISTS OVERLAPPING SUPPORTING BEAM



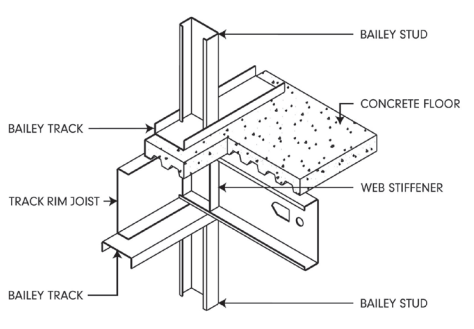
JOIST PERPENDICULAR TO PERIMETER



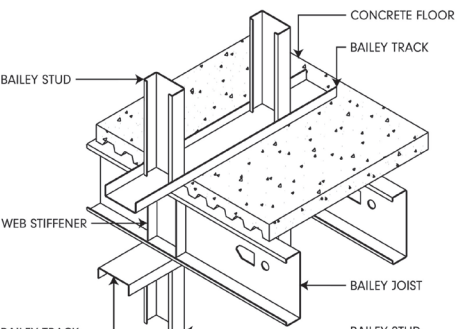
JOIST ABOVE LOADBEARING WALL



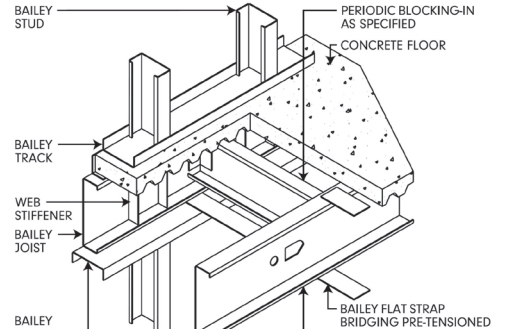
JOIST PERPENDICULAR TO PERIMETER



JOIST ABOVE LOADBEARING WALL



JOIST PARALLEL TO PERIMETER



# FLOOR JOIST SPAN LIMITATIONS

Joist Member	Yield Strength Fy(ksi)	15 psf Dead Load 40 psf Live Load					
		TL Deflection = L/240, LL Deflection = L/360			TL Deflection = L/240, LL Deflection = L/480		
		Spacing (in) o.c.			Spacing (in) o.c.		
		12	16	24	12	16	24
800S162-43	33	14.9	12.9	10.6	14.9	12.9	10.6
800S162-54	50	18.1	16.5	14.1	16.5	14.9	13.1
800S162-68	50	19.6	17.8	15.6	17.8	16.2	14.2
800S162-97	50	21.8	19.8	17.4	19.8	18	15.7
800S200-43	33	16	13.8	11.3	16	13.8	11.3
800S200-54	50	19.1	17.4	15.1	17.5	15.8	13.8
800S200-68	50	20.6	18.7	16.4	18.7	17	14.8
800S200-97	50	23	20.8	18.1	20.8	18.9	16.5
800S250-43	33	16.4	14.2	11.6	16.4	14.2	11.6
800S250-54	50	19.8	18	15.4	17.9	16.3	14.2
800S250-68	50	21.5	19.5	17	19.5	17.7	15.5
800S250-97	50	24	21.8	19	21.8	19.8	17.2
800S300-43	33	16.6	14.4	11.8	16.6	14.4	11.8
800S300-54	50	20.2	18.3	15.6	18.4	16.7	14.6
800S300-68	50	22	20	17.4	19.9	18.1	15.8
800S300-97	50	24.8	22.5	19.6	22.5	20.4	17.8
1000S162-54	50	21.6	19.1	15.6	19.6	17.8	15.6
1000S162-68	50	23.4	21.3	18.4	21.3	19.3	16.8
1000S162-97	50	26.4	24	20.8	23.9	21.8	18.9
1000S200-54	50	22.6	20.5	16.9	20.5	18.6	16.2
1000S200-68	50	24.4	22.2	19.5	22.2	20.2	17.6
1000S200-97	50	27.5	25	21.8	24.9	22.7	19.8
1000S250-54	50	23.6	21.2	17.3	21.4	19.5	17
1000S250-68	50	25.6	23.2	20.3	23.2	21	18.5
1000S250-97	50	28.7	26	22.8	26.1	23.6	20.7
1000S300-54	50	24	21.6	17.5	21.8	19.8	17.2
1000S300-68	50	26.2	23.8	20.7	23.8	21.6	18.8
1000S300-97	50	29.5	26.8	23.4	26.8	24.4	21.3
1200S162-68	50	27	24.5	20	24.5	22.3	19.5
1200S162-97	50	30.5	27.8	24.2	27.7	25.2	22
1200S200-68	50	28.2	25.6	21.6	25.6	23.2	20.3
1200S200-97	50	32	29	25.2	28.9	26.2	22.9
1200S250-68	50	29	26.5	22.3	26.4	24	20.9
1200S250-97	50	33	30	26.2	30.1	27.3	23.9
1200S300-68	50	30.2	27.5	22.7	27.6	24.9	21.8
1200S300-97	50	34.2	31	27	31.1	28.2	24.6
1400S162-68	50	30	26	21.1	27.6	25.1	21.1
1400S162-97	50	34.6	31.5	27.5	31.5	28.6	24.9
1400S200-68	50	31.7	28.2	23.1	28.7	26.1	22.8
1400S200-97	50	36	32.7	28.6	32.7	29.7	25.9
1400S250-68	50	32.7	29.4	24	29.7	26.9	23.6
1400S250-97	50	37.3	34	29.7	33.9	30.9	26.9
1400S300-68	50	33.3	30	24.5	30.3	27.6	24
1400S300-97	50	38.3	34.7	30.5	34.9	31.7	27.6

1. Spans are based on continuous support of compression flange over the full length of the joist.
2. Joist deflection limitations are based upon L/240 for the total load (TL) and L/360 or L/480 for live load (LL).
3. Joists must be braced against rotation at all supports.
4. The yield stress (33ksi or 50ksi) used to calculate tabulated values is indicated in each table.
5. Span tables are calculated per CSSBI 58-2018 Lightweight Steel Framing Member Selection Tables.
6. Stud distortional buckling based on an assumed  $K\phi=0$ .
7. Web punchouts are not considered for shear.
8. The tables do not include web crippling calculations. Web crippling and web stiffeners requirements to be designed as per CSSBI 58-2018 and Section G-5 of S136-16.

# FLOOR JOIST SPAN LIMITATIONS

Joist Member	Yield Strength Fy(ksi)	25 psf Dead Load 40 psf Live Load					
		TL Deflection = L/240, LL Deflection = L/360			TL Deflection = L/240, LL Deflection = L/480		
		Spacing (in) o.c.			Spacing (in) o.c.		
		12	16	24	12	16	24
800S162-43	33	13.9	12	9.8	13.9	12	9.8
800S162-54	50	17.7	15.9	13	16.5	14.9	13
800S162-68	50	19.1	17.3	15.2	17.8	16.2	14.2
800S162-97	50	21.2	19.3	16.9	19.8	18	15.7
800S200-43	33	14.9	12.9	10.5	14.9	12.9	10.5
800S200-54	50	18.7	16.9	14	17.5	15.8	13.8
800S200-68	50	20.1	18.2	15.9	18.7	17	14.8
800S200-97	50	22.3	20.2	17.7	20.8	18.9	16.5
800S250-43	33	15.3	13.2	10.8	15.3	13.2	10.8
800S250-54	50	19.2	17.5	14.3	17.9	16.3	14.2
800S250-68	50	20.9	18.9	16.5	19.5	17.7	15.5
800S250-97	50	23.3	21.2	18.5	21.8	19.8	17.2
800S300-43	33	15.4	13.4	10.9	15.4	13.4	10.9
800S300-54	50	19.6	17.7	14.5	18.4	16.7	14.5
800S300-68	50	21.4	19.4	16.9	19.9	18.1	15.8
800S300-97	50	24.1	21.9	19.1	22.5	20.4	17.8
1000S162-54	50	20.5	17.7	14.5	19.6	17.7	14.5
1000S162-68	50	22.7	20.7	17.1	21.3	19.3	16.8
1000S162-97	50	25.7	23.3	20.3	23.9	21.8	18.9
1000S200-54	50	21.9	19.1	15.7	20.5	18.6	15.7
1000S200-68	50	23.7	21.6	18.4	22.2	20.2	17.6
1000S200-97	50	26.7	24.3	21.2	24.9	22.7	19.8
1000S250-54	50	22.8	19.7	16.1	21.4	19.5	16.1
1000S250-68	50	24.9	22.6	18.9	23.2	21	18.5
1000S250-97	50	27.9	25.3	22.1	26.1	23.6	20.7
1000S300-54	50	23.1	19.9	16.3	21.8	19.8	16.3
1000S300-68	50	25.5	23.1	19.3	23.8	21.6	18.8
1000S300-97	50	28.7	26.1	22.7	26.8	24.4	21.3
1200S162-68	50	26.2	22.7	18.5	24.5	22.3	18.5
1200S162-97	50	29.7	27.1	23.6	27.7	25.2	22
1200S200-68	50	27.4	24.5	20	25.6	23.2	20
1200S200-97	50	31.1	28.1	24.6	28.9	26.2	22.9
1200S250-68	50	28.3	25.5	20.7	26.4	24	20.7
1200S250-97	50	32.1	29.3	25.5	30.1	27.3	23.9
1200S300-68	50	29.5	25.8	21	27.6	24.9	21
1200S300-97	50	33.2	30.1	26.3	31.1	28.2	24.6
1400S162-68	50	27.8	24.1	19.7	27.6	24.1	19.7
1400S162-97	50	33.6	30.6	25.8	31.5	28.6	24.9
1400S200-68	50	30.4	26.2	21.4	28.7	26.1	21.4
1400S200-97	50	35.1	31.9	27.8	32.7	29.7	25.9
1400S250-68	50	31.6	27.2	22.3	29.7	26.9	22.3
1400S250-97	50	36.4	32.9	28.8	33.9	30.9	26.9
1400S300-68	50	32.2	27.9	22.8	30.3	27.6	22.8
1400S300-97	50	37.2	33.9	29.5	34.9	31.7	27.6

1. Spans are based on continuous support of compression flange over the full length of the joist.
2. Joist deflection limitations are based upon L/240 for the total load (TL) and L/360 or L/480 for live load (LL).
3. Joists must be braced against rotation at all supports.
4. The yield stress (33ksi or 50ksi) used to calculate tabulated values is indicated in each table.
5. Span tables are calculated per CSSBI 58-2018 Lightweight Steel Framing Member Selection Tables.
6. Stud distortional buckling based on an assumed  $K\phi=0$ .
7. Web punchouts are not considered for shear.
8. The tables do not include web crippling calculations. Web crippling and web stiffeners requirements to be designed as per CSSBI 58-2018 and Section G-5 of S136-16.

# FLOOR JOIST SPAN LIMITATIONS

Joist Member	Yield Strength Fy(ksi)	35 psf Dead Load 40 psf Live Load					
		TL Deflection = L/240, LL Deflection = L/360			TL Deflection = L/240, LL Deflection = L/480		
		Spacing (in) o.c.			Spacing (in) o.c.		
		12	16	24	12	16	24
800S162-43	33	13	11.3	9.2	13	11.3	9.2
800S162-54	50	16.9	15	12.3	16.5	14.9	12.3
800S162-68	50	18.2	16.6	14.3	17.8	16.2	14.2
800S162-97	50	20.2	18.5	16.1	19.8	18	15.7
800S200-43	33	13.9	12	9.8	13.9	12	9.8
800S200-54	50	17.8	16.1	13.1	17.5	15.8	13.1
800S200-68	50	19.1	17.4	15.2	18.7	17	14.8
800S200-97	50	21.2	19.3	16.9	20.8	18.9	16.5
800S250-43	33	14.3	12.4	10.1	14.3	12.4	10.1
800S250-54	50	18.3	16.5	13.4	17.9	16.3	13.4
800S250-68	50	19.8	18.1	15.8	19.5	17.7	15.5
800S250-97	50	22.2	20.2	17.6	21.8	19.8	17.2
800S300-43	33	14.5	12.6	10.3	14.5	12.6	10.3
800S300-54	50	18.7	16.7	13.6	18.4	16.7	13.6
800S300-68	50	20.4	18.5	16	19.9	18.1	15.8
800S300-97	50	23	20.8	18.2	22.5	20.4	17.8
1000S162-54	50	19.3	16.7	13.6	19.3	16.7	13.6
1000S162-68	50	21.7	19.7	16.1	21.3	19.3	16.1
1000S162-97	50	24.5	22.2	19.4	23.9	21.8	18.9
1000S200-54	50	20.8	18	14.7	20.5	18	14.7
1000S200-68	50	22.7	20.7	17.2	22.2	20.2	17.2
1000S200-97	50	25.5	23.1	20.2	24.9	22.7	19.8
1000S250-54	50	21.4	18.5	15.1	21.4	18.5	15.1
1000S250-68	50	23.8	21.5	17.7	23.2	21	17.7
1000S250-97	50	26.6	24.1	21.1	26.1	23.6	20.7
1000S300-54	50	21.7	18.7	15.3	21.7	18.7	15.3
1000S300-68	50	24.3	22.1	18	23.8	21.6	18
1000S300-97	50	27.4	25	21.7	26.8	24.4	21.3
1200S162-68	50	24.6	21.3	17.4	24.5	21.3	17.4
1200S162-97	50	28.4	25.8	22.5	27.7	25.2	22
1200S200-68	50	26.1	23	18.8	25.6	23	18.8
1200S200-97	50	29.6	26.8	23.5	28.9	26.2	22.9
1200S250-68	50	27	23.9	19.5	26.4	23.9	19.5
1200S250-97	50	30.8	28	24.4	30.1	27.3	23.9
1200S300-68	50	28	24.2	19.8	27.6	24.2	19.8
1200S300-97	50	31.8	28.8	25.1	31.1	28.2	24.6
1400S162-68	50	26.1	22.6	18.5	26.1	22.6	18.5
1400S162-97	50	32.2	29.2	24.1	31.5	28.6	24.1
1400S200-68	50	28.4	24.6	20.1	28.4	24.6	20.1
1400S200-97	50	33.5	30.4	26.1	32.7	29.7	25.9
1400S250-68	50	29.6	25.6	20.9	29.6	25.6	20.9
1400S250-97	50	34.7	31.6	27	33.9	30.9	26.9
1400S300-68	50	30.2	26.1	21.4	30.2	26.1	21.4
1400S300-97	50	35.6	32.4	27.7	34.9	31.7	27.6

1. Spans are based on continuous support of compression flange over the full length of the joist.
2. Joist deflection limitations are based upon L/240 for the total load (TL) and L/360 or L/480 for live load (LL).
3. Joists must be braced against rotation at all supports.
4. The yield stress (33ksi or 50ksi) used to calculate tabulated values is indicated in each table.
5. Span tables are calculated per CSSBI 58-2018 Lightweight Steel Framing Member Selection Tables.
6. Stud distortional buckling based on an assumed  $K\phi=0$ .
7. Web punchouts are not considered for shear.
8. The tables do not include web crippling calculations. Web crippling and web stiffeners requirements to be designed as per CSSBI 58-2018 and Section G-5 of S136-16.

# FLOOR JOIST SPAN LIMITATIONS

Joist Member	Yield Strength F <sub>y</sub> (ksi)	15 psf Dead Load 100 psf Live Load					
		TL Deflection = L/240, LL Deflection = L/360			TL Deflection = L/240, LL Deflection = L/480		
		Spacing (in) o.c.			Spacing (in) o.c.		
		12	16	24	12	16	24
800S162-43	33	10.2	8.8	0	10.2	8.8	0
800S162-54	50	13.4	11.8	9.6	12.2	11	9.6
800S162-68	50	14.5	13.2	11.2	13.2	11.9	10.5
800S162-97	50	16.1	14.7	12.8	14.7	13.3	11.6
800S200-43	33	10.9	9.5	0	10.9	9.5	0
800S200-54	50	14.2	12.6	10.3	12.9	11.7	10.2
800S200-68	50	15.2	13.8	12	13.8	12.6	10.9
800S200-97	50	16.9	15.4	13.5	15.4	13.9	12.2
800S250-43	33	11.2	9.7	0	11.2	9.7	0
800S250-54	50	14.6	12.9	10.6	13.2	12	10.5
800S250-68	50	15.8	14.4	12.3	14.4	13	11.4
800S250-97	50	17.7	16.1	14	16.1	14.6	12.7
800S300-43	33	11.4	9.9	0	11.4	9.9	0
800S300-54	50	14.9	13.1	10.7	13.6	12.3	10.7
800S300-68	50	16.2	14.7	12.6	14.7	13.4	11.7
800S300-97	50	18.3	16.6	14.5	16.6	15.1	13.2
1000S162-54	50	15.1	13.1	10.7	14.5	13.1	10.7
1000S162-68	50	17.3	15.4	12.6	15.7	14.3	12.5
1000S162-97	50	19.4	17.7	15.4	17.6	16	14
1000S200-54	50	16.3	14.1	11.5	15.1	13.7	11.5
1000S200-68	50	18	16.4	13.6	16.4	14.9	13
1000S200-97	50	20.3	18.5	16.1	18.4	16.7	14.6
1000S250-54	50	16.7	14.5	11.8	15.8	14.4	11.8
1000S250-68	50	18.8	17	13.9	17.2	15.6	13.6
1000S250-97	50	21.1	19.2	16.8	19.2	17.5	15.3
1000S300-54	50	17	14.7	12	16.1	14.6	12
1000S300-68	50	19.3	17.3	14.1	17.5	15.9	13.9
1000S300-97	50	21.8	19.8	17.3	19.8	18	15.7
1200S162-68	50	19.3	16.7	13.7	18.1	16.4	13.7
1200S162-97	50	22.6	20.5	17.7	20.5	18.6	16.3
1200S200-68	50	20.8	18.1	14.8	18.8	17.1	14.8
1200S200-97	50	23.5	21.4	18.7	21.4	19.4	16.9
1200S250-68	50	21.5	18.7	15.3	19.5	17.7	15.3
1200S250-97	50	24.4	22.2	19.4	22.2	20.1	17.6
1200S300-68	50	22	19	15.6	20.3	18.4	15.6
1200S300-97	50	25.2	22.9	20	22.9	20.8	18.2
1400S162-68	50	20.5	17.8	14.5	20.4	17.8	14.5
1400S162-97	50	25.5	23.2	18.9	23.2	21.1	18.4
1400S200-68	50	22.3	19.3	15.8	21.2	19.3	15.8
1400S200-97	50	26.6	24.1	20.4	24.1	21.9	19.1
1400S250-68	50	23.2	20.1	16.4	21.9	19.9	16.4
1400S250-97	50	27.5	25	21.2	25	22.7	19.8
1400S300-68	50	23.7	20.5	16.7	22.3	20.3	16.7
1400S300-97	50	28.3	25.7	21.7	25.7	23.3	20.4

1. Spans are based on continuous support of compression flange over the full length of the joist.
2. Joist deflection limitations are based upon L/240 for the total load (TL) and L/360 or L/480 for live load (LL).
3. Joists must be braced against rotation at all supports.
4. The yield stress (33ksi or 50ksi) used to calculate tabulated values is indicated in each table.
5. Span tables are calculated per CSSBI 58-2018 Lightweight Steel Framing Member Selection Tables.
6. Stud distortional buckling based on an assumed K<sub>φ</sub>=0.
7. Web punchouts are not considered for shear.
8. The tables do not include web crippling calculations. Web crippling and web stiffeners requirements to be designed as per CSSBI 58-2018 and Section G-5 of S136-16.

# FLOOR JOIST SPAN LIMITATIONS

Joist Member	Yield Strength Fy(ksi)	25 psf Dead Load 100 psf Live Load					
		TL Deflection = L/240, LL Deflection = L/360			TL Deflection = L/240, LL Deflection = L/480		
		Spacing (in) o.c.			Spacing (in) o.c.		
		12	16	24	12	16	24
800S162-43	33	9.8	8.5	0	9.8	8.5	0
800S162-54	50	13.1	11.4	9.3	12.2	11	9.3
800S162-68	50	14.5	13.2	10.8	13.2	11.9	10.5
800S162-97	50	16.1	14.7	12.8	14.7	13.3	11.6
800S200-43	33	10.6	9.1	0	10.6	9.1	0
800S200-54	50	14.1	12.2	9.9	12.9	11.7	9.9
800S200-68	50	15.2	13.8	11.6	13.8	12.6	10.9
800S200-97	50	16.9	15.4	13.5	15.4	13.9	12.2
800S250-43	33	10.8	9.4	0	10.8	9.4	0
800S250-54	50	14.4	12.5	10.2	13.2	12	10.2
800S250-68	50	15.8	14.4	11.9	14.4	13	11.4
800S250-97	50	17.7	16.1	14	16.1	14.6	12.7
800S300-43	33	10.9	9.5	0	10.9	9.5	0
800S300-54	50	14.6	12.6	10.3	13.6	12.3	10.3
800S300-68	50	16.2	14.7	12.1	14.7	13.4	11.7
800S300-97	50	18.3	16.6	14.5	16.6	15.1	13.2
1000S162-54	50	14.6	12.6	10.3	14.5	12.6	10.3
1000S162-68	50	17.2	14.9	12.2	15.7	14.3	12.2
1000S162-97	50	19.4	17.7	15.4	17.6	16	14
1000S200-54	50	15.7	13.6	11.1	15.1	13.6	11.1
1000S200-68	50	18	16	13.1	16.4	14.9	13
1000S200-97	50	20.3	18.5	16.1	18.4	16.7	14.6
1000S250-54	50	16.2	14	11.3	15.8	14	11.3
1000S250-68	50	18.8	16.4	13.4	17.2	15.6	13.4
1000S250-97	50	21.1	19.2	16.8	19.2	17.5	15.3
1000S300-54	50	16.4	14.2	11.4	16.1	14.2	11.4
1000S300-68	50	19.3	16.7	13.7	17.5	15.9	13.7
1000S300-97	50	21.8	19.8	17.3	19.8	18	15.7
1200S162-68	50	18.6	16.1	13.2	18.1	16.1	13.2
1200S162-97	50	22.6	20.5	17	20.5	18.6	16.3
1200S200-68	50	20.2	17.5	14.2	18.8	17.1	14.2
1200S200-97	50	23.5	21.4	18.3	21.4	19.4	16.9
1200S250-68	50	20.8	18	14.7	19.5	17.7	14.7
1200S250-97	50	24.4	22.2	18.9	22.2	20.1	17.6
1200S300-68	50	21.2	18.4	15	20.3	18.4	15
1200S300-97	50	25.2	22.9	19.4	22.9	20.8	18.2
1400S162-68	50	19.8	17.1	0	19.8	17.1	0
1400S162-97	50	25.5	22.4	18.3	23.2	21.1	18.3
1400S200-68	50	21.5	18.6	15.2	21.2	18.6	15.2
1400S200-97	50	26.6	24.1	19.7	24.1	21.9	19.1
1400S250-68	50	22.4	19.4	15.8	21.9	19.4	15.8
1400S250-97	50	27.5	25	20.5	25	22.7	19.8
1400S300-68	50	22.9	19.8	16.2	22.3	19.8	16.2
1400S300-97	50	28.3	25.7	20.9	25.7	23.3	20.4

1. Spans are based on continuous support of compression flange over the full length of the joist.
2. Joist deflection limitations are based upon L/240 for the total load (TL) and L/360 or L/480 for live load (LL).
3. Joists must be braced against rotation at all supports.
4. The yield stress (33ksi or 50ksi) used to calculate tabulated values is indicated in each table.
5. Span tables are calculated per CSSBI 58-2018 Lightweight Steel Framing Member Selection Tables.
6. Stud distortional buckling based on an assumed  $K\phi=0$ .
7. Web punchouts are not considered for shear.
8. The tables do not include web crippling calculations. Web crippling and web stiffeners requirements to be designed as per CSSBI 58-2018 and Section G-5 of S136-16.



# FLOOR JOIST SPAN LIMITATIONS

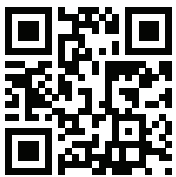
Joist Member	Yield Strength F <sub>y</sub> (ksi)	35 psf Dead Load 100 psf Live Load					
		TL Deflection = L/240, LL Deflection = L/360			TL Deflection = L/240, LL Deflection = L/480		
		Spacing (in) o.c.			Spacing (in) o.c.		
		12	16	24	12	16	24
800S162-43	33	9.5	8.3	0	9.5	8.3	0
800S162-54	50	12.7	11	8.9	12.2	11	8.9
800S162-68	50	14.5	12.8	10.5	13.2	11.9	10.5
800S162-97	50	16.1	14.7	12.8	14.7	13.3	11.6
800S200-43	33	10.2	8.8	0	10.2	8.8	0
800S200-54	50	13.6	11.8	9.6	12.9	11.7	9.6
800S200-68	50	15.2	13.8	11.2	13.8	12.6	10.9
800S200-97	50	16.9	15.4	13.5	15.4	13.9	12.2
800S250-43	33	10.5	9.1	0	10.5	9.1	0
800S250-54	50	13.9	12	9.8	13.2	12	9.8
800S250-68	50	15.8	14.1	11.5	14.4	13	11.4
800S250-97	50	17.7	16.1	14	16.1	14.6	12.7
800S300-43	33	10.6	9.2	0	10.6	9.2	0
800S300-54	50	14.1	12.2	10	13.6	12.2	10
800S300-68	50	16.2	14.3	11.7	14.7	13.4	11.7
800S300-97	50	18.3	16.6	14.5	16.6	15.1	13.2
1000S162-54	50	14.1	12.2	10	14.1	12.2	10
1000S162-68	50	16.6	14.4	11.8	15.7	14.3	11.8
1000S162-97	50	19.4	17.7	15	17.6	16	14
1000S200-54	50	15.2	13.2	10.7	15.1	13.2	10.7
1000S200-68	50	17.9	15.5	12.6	16.4	14.9	12.6
1000S200-97	50	20.3	18.5	16.1	18.4	16.7	14.6
1000S250-54	50	15.6	13.5	10.9	15.6	13.5	10.9
1000S250-68	50	18.4	15.9	13	17.2	15.6	13
1000S250-97	50	21.1	19.2	16.6	19.2	17.5	15.3
1000S300-54	50	15.8	13.7	10.9	15.8	13.7	10.9
1000S300-68	50	18.7	16.2	13.2	17.5	15.9	13.2
1000S300-97	50	21.8	19.8	16.9	19.8	18	15.7
1200S162-68	50	18	15.6	12.7	18	15.6	12.7
1200S162-97	50	22.6	20.2	16.5	20.5	18.6	16.3
1200S200-68	50	19.5	16.9	13.8	18.8	16.9	13.8
1200S200-97	50	23.5	21.4	17.7	21.4	19.4	16.9
1200S250-68	50	20.2	17.5	14.3	19.5	17.5	14.3
1200S250-97	50	24.4	22.2	18.3	22.2	20.1	17.6
1200S300-68	50	20.5	17.7	14.5	20.3	17.7	14.5
1200S300-97	50	25.2	22.9	18.7	22.9	20.8	18.2
1400S162-68	50	19.1	16.6	0	19.1	16.6	0
1400S162-97	50	25	21.7	17.7	23.2	21.1	17.7
1400S200-68	50	20.8	18	14.7	20.8	18	14.7
1400S200-97	50	26.6	23.4	19.1	24.1	21.9	19.1
1400S250-68	50	21.6	18.7	15.3	21.6	18.7	15.3
1400S250-97	50	27.5	24.2	19.8	25	22.7	19.8
1400S300-68	50	22.1	19.2	15.4	22.1	19.2	15.4
1400S300-97	50	28.3	24.8	20.3	25.7	23.3	20.3

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6. Stud distortional buckling based on an assumed  $K\phi=0$ .
7. Web punchouts are not considered for shear.
8. The tables do not include web crippling calculations. Web crippling and web stiffeners requirements to be designed as per CSSBI 58-2018 and Section G-5 of S136-16.



# BAILEY<sup>®</sup>

METAL PRODUCTS LIMITED



## BAILEY C JOIST SPAN TABLES AND DETAILS

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